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Experimental Investigation of Thermal Volumetric Changes in Clays: Unveiling Hidden Controls --Manuscript Draft--

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Abstract:	This endeavour investigates the thermally induced volumetric behaviour of fine-grained soils, specifically focusing on the effects of over-consolidation ratio (OCR), plasticity, and stress history. A custom-designed temperature-controlled oedometer cell was used to conduct drained heating and heating-cooling cycle tests on silty clays from Budapest, Hungary. The results reveal that normally consolidated samples exhibit plastic contraction upon heating, while over-consolidated samples show varying responses influenced by their recent stress history and plasticity index. Overconsolidated soil samples with a recent stress history of loading (reloading) predominantly contracted rather than the traditionally expected initial expansion, highlighting the importance of recent stress paths. Plasticity significantly impacts volumetric strain, with higher-plasticity clays exhibiting more pronounced contractions during heating and greater cumulative plastic strain over thermal cycles. The study concludes that a combination of OCR, plasticity, and stress history collectively determine the thermal volume response of fine-grained soils, with implications for the design of thermally robust geo-energy infrastructures.		



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Dear Editors

February 3, 2025

I am pleased to submit our manuscript, entitled "Experimental Investigation of Thermal Volumetric Changes in Clays: Unveiling Hidden Controls," for consideration in *Engineering Geology*.

Given the journal's focus on geohazards, site characterization, tunnel engineering, and other related domains, we believe our work offers valuable insights into the thermal-mechanical processes of clayey soils. Our research addresses both engineering geology and geotechnical engineering concerns, showing how key parameters—over-consolidation ratio (OCR), plasticity index (PI), and stress history—dictate the behavior of fine-grained soils under thermo-mechanical loads. We present an experimental program especially relevant to:

- o Geohazards and Risk Assessment: Understanding how clayer layers respond to heat is essential in regions where tunnels, underground storage facilities, or geothermal systems may face elevated temperatures, affecting both slope stability and site characterization
- Site Characterization and Thermo-Hydro-Mechanical (THM) Behavior: Our temperature-controlled oedometer tests provide new data on how stress history influences volumetric changes, linking to THM properties crucial in landslide assessments and rock-hazard evaluations.
- O Time-Dependent Processes in Clays: We show how repeated heating—cooling cycles can lead to either progressive or stabilized strains, relevant to the long-term performance of geological barriers and soils exposed to fluctuating subsurface temperatures.

We have modified our manuscript to align with the standards of Engineering Geology,

including an expanded geological background, their mineralogy, and the implications for landslides, hazard assessment, and site investigations. We illustrate how our findings can guide *risk assessment* for geothermal installations and underground storage facilities, as well as *long-term THM behavior* relevant to barriers and reservoirs.

Thank you for your time and consideration. We appreciate the opportunity to have our manuscript reviewed by experts whose research aligns closely with our work in geohazards, engineering geology mapping, risk and reliability, and anisotropic behavior of soils and rocks. We are confident that our research will interest readers seeking to address geotechnical and geological challenges in thermally active subsurface environments.

If you have any questions, please do not hesitate to contact me. We look forward to your feedback.

Sincerely

Saeed Tourchi, PhD

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¹ Highlights

- 2 Experimental Investigation of Thermal Volumetric Changes in Clays:
- 3 Unveiling Hidden Controls
- 4 Hamed HoseiniMighani, Saeed Tourchi, Arash Alimardani Lavasan, Fate-
- 5 mehsadat Hosseini, Janos Szendefy
- Stress history dominates thermal volume change in over-consolidated
 clays.
- High-plasticity clays show larger thermal strains than low-plasticity
 soils.
- Soil fabric evolves toward elastic behavior under repeated heating-cooling cycles.
- Distinct relations identified between volumetric strain, OCR, and plasticity.
- Findings enhance the design of thermally resilient geo-energy and waste disposal.
- Findings improve risk assessments for geothermal and underground storage projects.

Experimental Investigation of Thermal Volumetric Changes in Clays: Unveiling Hidden Controls

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7 Abstract

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Understanding the thermally induced volumetric behavior of fine-grained soils is central to designing resilient geo-energy infrastructures, including geothermal systems, underground storage facilities, and deep repositories for radioactive waste. In this study, we investigate the coupled thermal-mechanical response of silty clays from Budapest, Hungary, characterized by contrasting plasticity and stress histories. Drained heating and heating–cooling cycle tests were performed in a custom-built, temperature controlled oedometer on both low-plasticity (LP) and high-plasticity (HP) samples. Temperatures ranged from 20°C to 90°C under variable over-consolidation ratios (OCR = 1 to 22). Results confirm that normally consolidated clays exhibit consistent plastic contraction during thermal loading, whereas highly over-consolidated clays reveal a more nuanced response, largely governed by recent stress history. Significantly, reloading events led to contraction in

samples where a small elastic expansion might otherwise be expected. Highplasticity samples consistently showed greater thermal strain magnitudes,
underscoring the importance of the plasticity index in controlling volume
change. Repeated heating—cooling cycles induced mostly plastic strain during the first cycle, transitioning toward elastic behavior and stabilizing volumetric changes by the fourth. These findings offer deeper insight into the
mechanics of thermally affected clay deposits, with direct relevance to geological risk assessments for geothermal fields and to the engineered barriers
of nuclear waste repositories. By highlighting the pivotal roles of OCR, plasticity index, and stress history, this research contributes to more accurate
predictive models and more robust designs for thermally active subsurface
infrastructures.

Keywords: Drained Heating, Heating-Cooling Cycles, Fine-Grained Soils,

55 1. Introduction

Thermal and mechanical loads on fine-grained soils have become an increasingly crucial research focus in geo-energy systems. As infrastructure
must remain resilient in thermally active environments, it is essential to understand how temperature fluctuations and mechanical stresses affect soil
properties and long-term stability. Applications such as geothermal energy
systems (Laloui et al., 2006; Faizal et al., 2018; Ng et al., 2016; Aljundi et al.,
2024; Liu et al., 2024), nuclear waste repositories (Gens et al., 2009; Tourchi

Thermal-Induced Volumetric Changes, Clay, Geotechnical Engineering

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2017), and energy piles (Aresti et al., 2018; Liu and Xu, 2017) expose soils
to repeated heating and cooling, potentially altering their deformation char-
acteristics and structural integrity. Similarly, soil-atmosphere interactions
in temperate climates (Cui, 2022; Scaringi and Loche, 2022; Melchiorre and
Frattini, 2011) and fault zone processes during landslides (Scaringi et al.,
2022; Jabbarzadeh et al., 2024; Tourchi et al., 2024; Shibasaki et al., 2017;
Sadeghi et al., 2024; Tian et al., 2023; Gong et al., 2024) underscore the
broader impact of temperature variations on soil stability. Fine-grained soils,
such as clays and silts, are particularly sensitive due to their mineral compo-
sition and complex inter-particle interactions (Hoseinimighani and Szendefy,
2022; Hoseinimighani et al., 2023).
   Research on temperature-induced changes in soil behavior traces back
to foundational works by Gary (1936) and Paaswell (1967), who performed
oedometer tests under different thermal conditions. Early studies primarily
focused on modest temperature ranges (10–50°C) to account for laboratory-
to-field differences; however, modern engineering challenges require dealing
with much broader thermal variations, spanning time scales from seconds
to centuries. Consequently, current interest extends beyond conventional
geotechnical scenarios to applications such as deep geothermal energy ex-
traction, high-level nuclear waste disposal, and energy storage.
   A critical factor in understanding soil volume changes under thermal loads
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et al., 2021; Bumbieler et al., 2021; Armand et al., 2017; Ballarini et al.,

is the Over-Consolidation Ratio (OCR), defined as the ratio of a soil's past

maximum effective stress to its current effective stress. Under drained heating at constant effective stress, NC soils typically exhibit plastic volume contraction. In contrast, highly over-consolidated soils (OCR= 4-8) can show an elastic expansion volume response at lower temperatures before eventually contracting at higher temperatures, often at a transition temperature where expansion turns to plastic contraction (Towhata et al., 1993; Coccia and McCartney, 2016; Favero et al., 2016; Mohajerani et al., 2014). Another important distinction involves intact (undisturbed) soil samples, which preserve their natural structure and bonding, versus remolded samples reconstituted in the laboratory. Intact samples may respond differently under thermal stress due to the presence of in situ inter-particle bonds (Hueckel and Borsetto, 1990; Burghignoli et al., 2000; Hamidi et al., 2024).

Studies generally agree that higher OCR often correlates with a greater tendency for expansion prior to contraction and, in some cases, a higher transition temperature (AbuelNaga et al., 2006a; Baldi et al., 1988; Delage et al., 2000; Vega and McCartney, 2015). Thermally induced changes are in-101 fluenced by alterations in water viscosity, inter-particle forces, and structural 102 rearrangements within the soil matrix (Hueckel and Borsetto, 1990; Pothirak-103 sanon et al., 2010; Shetty et al., 2019). In addition, plasticity—governed 104 by clay mineralogy and water content—can modify the magnitude and rate 105 of volume changes; high-plasticity clays sometimes exhibit more pronounced initial expansion yet lower net contraction than low-plasticity clays (for overconsolidated soils) (Sultan et al., 2002; Chen et al., 2017). Overall, the literature highlights a multifaceted interaction of stress history, plasticity, and soil fabric in determining thermal volume behavior.

Despite these broad trends, there remain discrepancies that suggest the 111 influence of other critical factors. For example, Cekerevac and Laloui (2004) 112 found constant transition temperatures regardless of OCR, challenging the 113 notion that higher OCR consistently raises the transition point. Hueckel and 114 Borsetto (1990) observed that an intact clay with OCR = 5.7 transitioned at 115 a lower temperature than a remolded clay with OCR = 2.5, indicating that natural structure and inter-particle bonds could outweigh the effects of stress history. Moreover, OCR sample preparation techniques (e.g., reloading ver-118 sus unloading)—referred to as recent stress history—significantly influence soil behavior. Unloaded samples exhibited expansion at lower temperatures, while reloaded samples primarily contracted upon heating (Towhata et al., 1993; Burghignoli et al., 2000). These unresolved issues hinder the development of universally accurate predictive models for thermally influenced geotechnical systems. 124

Despite extensive investigations into thermally induced volume changes,
this gap in understanding remains evident—especially in undisturbed finegrained soils where the interplay of stress history, plasticity, and natural
bonding has yet to be fully characterized. Consequently, the ability to design reliable, thermally robust foundations and containment systems is undermined by the lack of a comprehensive framework that captures these complex
processes. The present study aims to bridge this gap by systematically ex-

amining low-plasticity and high-plasticity clays from District 13 and District 8 in Budapest, Hungary, under controlled thermal and mechanical conditions in a custom temperature-controlled oedometer cell.

In particular, this study seeks to:

- 1. Investigate how OCR influences thermal expansion and contraction behaviors in fine clay soil samples.
- 2. Quantify the impact of plasticity on transition temperature and the extent of volumetric changes during drained heating.
- 3. Assess how recent stress history, OCR, plasticity, and thermal cycling jointly shape the soil's volume change mechanisms.

This research contributes to more robust predictive models by clarifying
the relative roles of stress history, plasticity, and stress history in determining thermally induced volume changes. Ultimately, the findings will inform
improved design strategies for infrastructure subjected to repeated heating
and cooling, including energy piles, geothermal projects, and nuclear waste
disposal facilities.

Temperature variations significantly affect clay behavior by altering consolidation, mineral-water interactions, and anisotropy. Selected formations
with high montmorillonite content show notable thermo-mechanical coupling,
where thermal loading alters void ratio, permeability, and shear strength.
Understanding these interactions is crucial for designing geo-energy systems,
as repeated thermal cycles can cause cumulative deformations and structural
failures. Findings from this study impact infrastructure development in Bu-

dapest and similar regions. Accurate geological assessments are essential for resilient structure planning, especially in thermally active areas. Insights from District 13 and District 8 soils can guide site selection, material suitability, and performance monitoring, ensuring sustainability and safety in geotechnical engineering projects.

Beyond the local stratigraphy of Budapest, thermal-induced volume changes in clayey formations carry broader geologic implications for geothermal energy installations and underground storage facilities. In geothermal reservoirs—particularly those tapping sedimentary basins with significant clay content—thermal fluctuations may alter porosity and permeability, thus affecting both the efficiency of heat extraction and the potential for induced seismicity. Similarly, for underground facilities storing energy, hydrocarbons, or even high-level nuclear waste, clay barriers or caprock layers are crucial for long-term containment.

Incorporating our experimental findings—specifically on the roles of overconsolidation ratio, plasticity index, and stress history—into the mentioned
geo-energy applications can lead to more reliable predictions of deformation
and potential fluid migration. Consequently, local site investigations (like
those in Districts 13 and 8 in Budapest) have broader geological utility by
contributing to refined risk assessments and improved management strategies
for thermally active subsurface projects worldwide.

6 2. Geological Background of Study Area

The study area comprises two distinct regions within Budapest, Hungary: 177 District 13 and District 8. Each region exhibits unique geological and lithological characteristics that significantly influence the engineering behavior of fine-grained soils. District 13 lies within the Pest Plain, a subdivision of the Great Hungarian Plain, and is positioned at the transition between Upper Pleistocene-Holocene fluvioeolian sands (fQh3-hh) and Holocene riverine silts 182 (fQh2al), as delineated by Hungary's Geological Map. The subsurface pro-183 file predominantly consists of sandy, silty clay deposits of Miocene age, with 184 minor intercalations of gravel streaks that reflect fluvial and aeolian trans-185 port processes. Soil sampling was conducted at a depth of 13.5 m, where the 186 upper layers, up to 16 m, exhibit a relatively loose structure transitioning 187 to a denser, more consolidated state with increasing depth. 188

District 8, located within the Pesti Alluvial Fan Microregion, is underlain by Upper Pleistocene-Holocene fluvioeolian sands (feQp3-hh), which predominantly consist of fine-grained sediments with occasional sandy interbeds.

The grey clay deposits sampled at a depth of 8.5 m correspond to Mioceneaged formations characterized by high plasticity and low permeability. These
clayey deposits suggest deposition under low-energy fluvial or lacustrine conditions, resulting in a cohesive matrix with significant consolidation history.

3. Material Properties

As described above, the soils used in this study were fine-grained samples collected from two distinct locations in Budapest, Hungary: District 13
and District 8. These areas represent different geological contexts, offering
insights into the behavior of low-plasticity and high-plasticity clays under
thermal and mechanical loads. The samples were extracted as undisturbed
cores to preserve their natural structure and properties.

District 13 samples (we call LP)—LPs were collected from a depth of 13.5 m, corresponding to a Miocene-age sandy silty clay layer. Classified as CL (low-plasticity clay) under the Unified Soil Classification System (USCS), its index properties are summarised in Table 1. Figure 1a displays the particle size distribution from multiple tests, emphasizing the variability in soil composition and measurements for a more accurate representation of the tested soil samples.

Table 1: Summary of index properties for LP sample

Property	Symbol	Unit	Value
Sat unit weight	γ	$\mathrm{kN/m^3}$	20.4-21.2
Water content	ω	%	18-22
Liquid limit	LL	%	41-46
Plasticity index	PΙ	%	16-21
Initial void ratio	e_0	_	0.52 - 0.57
Particle density	ρ_s	$\rm g/cm^3$	2.586-2.658

District 8 samples (we call HP samples)—HPs were collected from a depth of 8.5 m and identified as a highly plastic grey clay from a Miocene-aged layer. Classified as CH (high-plasticity clay) under USCS, its index properties are 213 also presented in Table 2. The particle size distribution for this sample is 214 provided in Figure 1.

Table 2: Summary of index properties for HP sample

Property	Symbol	Unit	Value
Unit weight	γ	$\mathrm{kN/m^3}$	19.9-20.8
Water content	ω	%	21-27
Liquid limit	$\mid ext{LL} \mid$	%	55-61
Plasticity index	PI	%	35-38
Initial void ratio	e_0	_	0.65 - 0.67
Particle density	ρ_s	$ m g/cm^3$	2.67

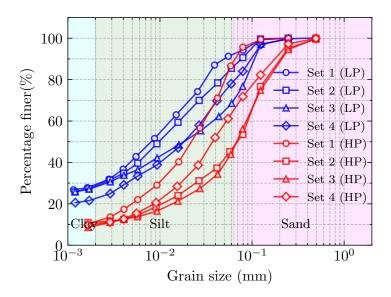


Figure 1: Particle distribution for Sample LP (A) and HP (B) samples.

The groundwater levels during sampling were consistent with the expected local geology. The LP sample's groundwater depth ranged between 6.8 and 9.5 meters, and the HP sample's was approximately 3.5 meters.

These data confirm the suitability of the chosen depths for evaluating the

behavior of natural, undisturbed clay layers.

220 4. Experimental Apparatus

The experimental program utilized a temperature-controlled oedometer apparatus specially designed for this study to investigate soil behavior under coupled thermal and mechanical loads. The conventional oedometer cell was modified to incorporate thermal regulation while preserving the core functionality of a standard consolidation setup.

The oedometer cell was surrounded by an electrical heater capable of pre-226 cise temperature adjustments. The heater was calibrated to deliver a uniform temperature gradient at a rate of 0.3°C/hour, which was critical for maintain-228 ing drained conditions and avoiding thermal pore pressure generation. The 229 apparatus was enclosed in an aluminum casing lined with insulating material to minimize heat loss and maintain thermal stability. Three Type K thermocouples were installed—one in contact with the heater and two submerged 232 in the water surrounding the soil sample—to accurately monitor and control 233 the test environment. This setup ensured uniform temperature distribution 234 throughout the test. A water sensor, integrated into the consolidation cell, maintained full saturation during elevated temperature phases by compensating for evaporation through automated water addition. Figure 2 illustrates 237 the modified oedometer cell and its components. Vertical stress was applied using a calibrated lever arm system, with weights placed at varying distances to achieve precise load increments. Vertical displacement was recorded continuously using a Linear Variable Differential Transformer (LVDT) with an accuracy of $1\,\mu\text{m}$. The LVDT was connected to a data acquisition system for real-time monitoring.



Figure 2: Modified thermal consolidation test device.

44 5. Experimental Program

The experimental program was designed to investigate the coupled thermal-mechanical behavior of fine-grained soils under drained heating and heating-cooling thermal cycles. The tests evaluated the role of over-consolidation ratio (OCR), plasticity, and stress history in shaping the volumetric response of natural clays.

250 Drained Heating Tests (h)

This phase of the experimental program focused on understanding the 251 influence of OCR on the volumetric behavior of natural clay samples. Undisturbed samples were first subjected to mechanical preloading to establish specific OCR conditions. These included a normally consolidated state (OCR = 1) and progressively over-consolidated states (OCR = 2, 6, and higher), achieved using controlled incremental loading and unloading processes. The preloading procedures ensured the stress history was systematically varied across the test specimens. The highly over-consolidated samples LP12 and 258 HP22 were prepared differently from the rest of the samples. OC Samples 259 were created using an unloading method. This involved loading the sample 260 and then unloading it to the target stress. 261

In contrast, samples LP12 and HP22 were not subjected to prior mechanical loading but were only loaded by the weight of the loading cap and
are thus not affected by prior unloading. This difference in preparation is
essential because the recent stress history of the soil (i.e., whether the sample

was loaded or unloaded) can influence its thermal response. While all OC samples experienced unloading as part of testing, samples LP12 and HP22 experienced a recent loading before the heating phase.

Once the desired OCR was achieved, the samples were subjected to thermal loading. The temperature was increased from 25°C to 85°C at a controlled rate of 0.3°C/hour, chosen to maintain drained conditions and avoid
the buildup of thermal pore pressures. The effective stress was held constant
throughout the heating process. Vertical displacements were continuously
recorded using a high-resolution Linear Variable Differential Transformer
(LVDT), and volumetric strain was calculated based on the monitored displacement. The mechanical and thermal paths for these tests are summarised
in Table 3, while Figure 3 provides a schematic representation of these paths.

Table 3: Summary of drained heating tests.

ID	Loading	OCR	Thermal Loading
LP1	$5 \rightarrow 1200$	1	$25 \rightarrow 85$
LP2	$5 \rightarrow 1200 \rightarrow 600$	2	$25 \rightarrow 85$
LP6	$5 \rightarrow 1200 \rightarrow 200$	6	$25 \rightarrow 85$
LP12	5	12	$25 \rightarrow 85$
HP1	$5 \rightarrow 800$	1	$25 \rightarrow 85$
HP2	$5 \rightarrow 1200 \rightarrow 600$	2	$25 \rightarrow 85$
HP6	$5 \rightarrow 1200 \rightarrow 200$	6	$25 \rightarrow 85$
HP22	5	22	$25 \rightarrow 85$

278 Heating-Cooling Thermal Cycle Tests (TC)

Heating-cooling thermal cycle tests were conducted to evaluate the cumulative effects of thermal cycling on the volumetric behavior of soils. These

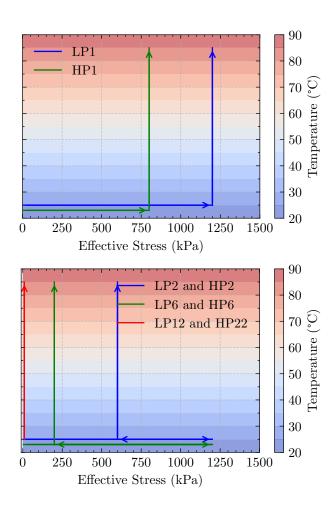


Figure 3: Mechanical and thermal paths for drained heating tests (AH1-BH1 and AH2, AH3, AH4).

tests aimed to distinguish between elastic and plastic deformation components and to assess the stabilization trends over multiple thermal cycles.

Samples were initially pre-loaded to specific OCR conditions, similar to those used in the drained heating tests, to ensure consistency in stress history. The thermal cycles alternated between 25°C and 85°C, with each heating and cooling phase conducted at a 0.3°C/hour rate. This controlled thermal rate ensured drained conditions and prevented excess pore pressure development during cycling.

At the end of each heating-cooling cycle, volumetric changes were recorded to evaluate the progressive deformation characteristics of the soils.

The thermal paths and loading protocols for these tests are detailed in Table 4, and a graphical depiction of the thermal cycling process is provided in Figure 4.

Table 4: Summary of drained heating-cooling tests.

ID	Loading	OCR	Thermal Loading
LP1	$5\rightarrow1200$	1	$25 \rightarrow 85 \rightarrow 25$
LP2	$5 \rightarrow 1200 \rightarrow 600$	2	$25 \rightarrow 85 \rightarrow 25$
LP6	$5 \rightarrow 1200 \rightarrow 200$	6	$25 \rightarrow 85 \rightarrow 25$
HP1	$5\rightarrow800$	1	$25 \rightarrow 85 \rightarrow 25$
HP2	$5 \rightarrow 1200 \rightarrow 600$	2	$25 \rightarrow 85 \rightarrow 25$
HP6	$5{\rightarrow}1200{\rightarrow}200$	6	$25 \rightarrow 85 \rightarrow 25$

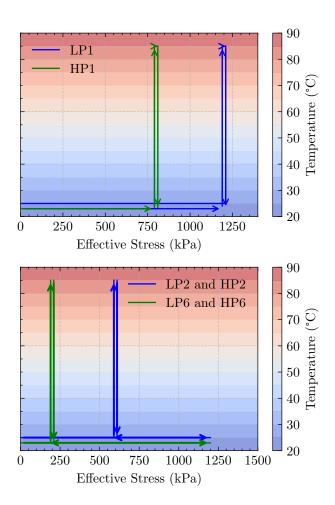


Figure 4: Mechanical and thermal paths for heating-cooling cycle tests.

6. Results and Discussion

95 6.1. Drained Heating Tests

change.

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The thermal volumetric response of selected samples under slow-drained 296 heating is examined in Figures 5a and 5b, which depict volumetric strain 297 versus temperature increment for LP samples and HP samples, respectively. Normally, consolidated samples consistently exhibit contraction during drained heating for both LP and HP samples. This behavior aligns with prior studies (e.g., AbuelNaga et al., 2006b; Baldi et al., 1988). Previous studies on slightly over-consolidated clay samples, including Pontida silty clay (OCR=2) (Baldi et al., 1988), Kaolin clay (OCR=2) (Cekerevac and Laloui, 2004), and Pasquasia clay (OCR=3.4) (Hueckel and 304 Borsetto, 1990), consistently exhibit purely contractive thermal responses. These behaviors align closely with the observations from samples LP2 and 306 HP2 in this study, reinforcing the understanding that clays with moderate 307 over-consolidation ratios (OCR \leq 3.4) primarily experience volume contrac-308 tion when subjected to heating. 309 However, all tested OC samples displayed contraction—a limited initial 310 expansion was observed for HP samples with OCR = 6 and 22, which quickly 311 diminished and transitioned to contraction. These measurements deviate 312 from previous findings, which generally show that highly over-consolidated 313 clay samples typically undergo an initial expansion, followed by contraction. 314 This implies that OCR alone may be insufficient to predict thermal volume

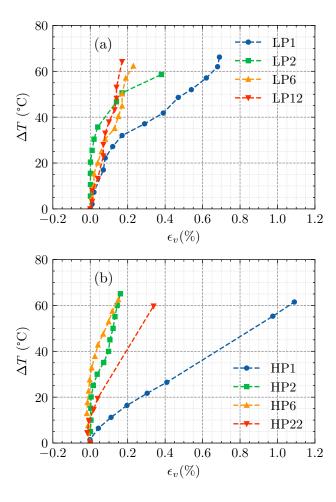


Figure 5: Thermal volume change after drained heating for (a) Low Plasticity (LP) and (b) High Plasticity (HP) samples, illustrating the predominant contraction behavior across different OCRs.

Samples with an OCR = 6 exhibited distinct thermal volumetric behaviors directly influenced by their plasticity levels. Specifically, the lowplasticity sample (LP6) demonstrated significant and consistent contraction
during thermal loading, lacking the initial expansion typically expected for
this OCR level. In contrast, the high-plasticity sample (HP6) showed only a
minimal initial expansion before transitioning to contraction. This deviation
from conventional expectations can be attributed to the presence of sand
grains within the soil matrix.

Thermal contraction observed in fine soils occurs due to several interrelated mechanisms. Studies have shown that thermally induced pore pressure
dissipation under drained conditions leads to volumetric contraction (Delage et al., 2000). Additionally, elevated temperatures reduce the thickness
of the double layer, potentially causing particle rearrangement and volume
contraction (Campanella and Mitchell, 1968). Another significant factor is
the reduction in the viscous shear resistance of pore water with increasing
temperature, which enhances particle mobility and structural collapse (Cui
et al., 2000).

Thermal expansion, on the other hand, is attributed partly to the thermal expansion of soil constituents (Campanella and Mitchell, 1968) and partly to increased inter-particle repulsion forces (Israelachvili, 1991; AbuelNaga et al., 2007b), which result in the expansion of inter-particle spacing and the formation of larger macropores. These thermal effects, acting synergistically, highlight the complex interplay between mechanical and physicochem-

ical processes in fine soils subjected to thermal loading.

The contraction observed in LP samples, and the lack of expansion in 341 over-consolidated samples suggest that the mineral composition and lowplasticity nature of LP soils dominate their thermal response. This results 343 in limited electrochemical interactions and lower water adsorption capacity. These characteristics reduce the tendency for inter-particle repulsion, allowing other mechanisms responsible for thermal contraction to dominate the thermal volume change. In contrast, HP samples with higher plasticity exhibit a greater capacity for physicochemical interactions and inter-particle repulsion forces. At increased temperatures, these forces amplify, leading to the initial expansion observed in over-consolidated states. However, as temperatures continue to rise, mechanisms such as the thermally induced reduction in double-layer thickness, the reduction in viscous shear resistance, and the dissipation of thermally induced excess pore pressure become dominant, resulting in contraction. This could explain the smaller and less pronounced thermal expansion observed in the over-consolidated HP sample compared to results reported in the literature. 356 The highly over-consolidated samples, namely LP12 (OCR=12) and 357

The highly over-consolidated samples, namely LP12 (OCR=12) and HP22 (OCR=22) were subjected to reloading before heating. The rationale for this approach is taken from the studies of Burghignoli et al. (2000)
and Towhata et al. (1993), which investigated the effects of different stress
histories—namely unloading and reloading—on the volumetric response of
soils under thermal loading. Our objective in exploring the impact of stress

history through various stress paths was to determine how prior mechanical loading (recent stress history) influences the thermal behavior of clayey soils. By applying unloading and reloading scenarios, we aimed to simulate real-world conditions and evaluate their impact on soil microstructure, void ratios, and volume changes during heating.

Figure 5 revealed NC and slightly over-consolidated samples contract 368 when heated under drained conditions, but some highly over-consolidated 369 samples depart from the classic *initial expansion* expectation. Figure 6 explains this discrepancy by comparing the normalized change in void ratio to 371 the over-consolidation ratio (OCR), incorporating data from both this study and previous work by Burghignoli et al. (2000) and Towhata et al. (1993). By scaling Δe_T by the temperature increment (ΔT) , Figure 6 demonstrates that recent stress history—involving unloading versus reloading—shapes the soil's thermal response as much as OCR itself. Normally, consolidated soils consistently contract, but overconsolidated samples can behave similarly if reloaded, showing minimal or no expansion. This confirms that OCR alone is not a definitive predictor of thermal volume change, highlighting the key 379 role of recent loading paths in governing soil behavior under heating. 380

Additionally, soil plasticity emerged as a significant factor. Samples with higher plasticity (HP1 and HP22) exhibited greater contraction under thermal loading compared to low-plasticity samples (LP1 and LP12). The mineral composition and microstructural properties of these clays facilitate more pronounced particle movement and rearrangement during temperature fluc-

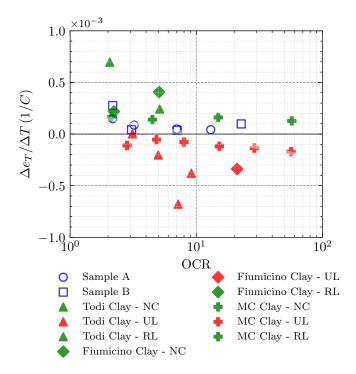


Figure 6: Thermally-induced volume change for Todi and Fiumicino clay (Burghignoli et al., 2000), MC clay (Towhata et al., 1993), and LP sample and B

tuations, making them more susceptible to sustained contractions. Moreover, the effect of plasticity on thermally induced volumetric strain is more
pronounced in samples not subjected to unloading, where constant stress
conditions allow for a clearer assessment of plastic deformation mechanisms,
uninfluenced by elastic rebound or stress-induced volume changes.

To demonstrate the relationship between thermally driven volumetric response and a soil's plasticity, Figure 7 illustrates how a soil's plasticity index (PI) correlates with the normalized thermally induced volumetric strain observed in normally consolidated samples during drained heating. The data exhibits a clear trend of exponentially increasing thermal strain with increasing plasticity index across all tested materials and datasets. This relationship can be modeled by the exponential equation:

$$\varepsilon_{v,T}/\Delta T(\%/C) = 0.0061 \cdot e^{0.378PI(\%)}$$
 (1)

where $\varepsilon_{v,T}/\Delta T$ represents the normalized volumetric strain, and PI is the plasticity index. This suggests that soils with higher PI values are more susceptible to greater volumetric changes during drained heating, indicating a strong, directly proportional relationship between plasticity and thermal response under specified conditions.

6.2. Heating-cooling thermal cycle (TC)

The thermal volume change of clay samples during the first heating and cooling is shown in Figures 8 for LP and HP samples. For LP samples, the

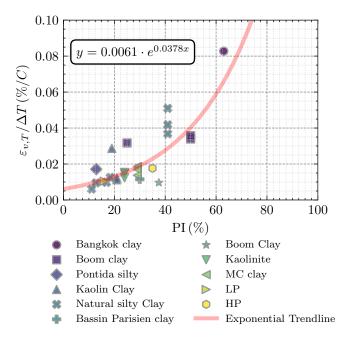


Figure 7: Effect of plasticity on volumetric strain after heating-cooling cycle for normally consolidated samples from literature and this research, including Bangkok clay (AbuelNaga et al., 2006a), Boom clay (Baldi et al., 1988, Robinet et al., 1997, Sultan et al., 2002), Pontida silty (Baldi et al., 1988), Kaolin Clay (Cekerevac and Laloui, 2004, Takai et al., 2016), Natural silty Clay (Di Donna and Laloui, 2015; Shetty et al., 2019), Illite (Plum and Esrig, 1969), Bassin Parisien clay (Robinet et al., 1997), Kaolinite (Shetty et al., 2019), and MC clay (Towhata et al., 1993).

single heating and cooling cycle caused contraction across all consolidation states. The normally consolidated samples exhibited significant plastic and 407 irreversible contraction. As the OCR increased, the extent of plastic contraction decreased notably. The thermal response for higher OCRs (OCR = 6) became primarily elastic, resulting in minimal contraction during the heating-cooling cycle. This progression demonstrates the stabilizing effect of over-consolidation, which enhances the resistance of soil structure to ther-412 mally induced deformation.

HP samples—with higher plasticity—demonstrated similar trends but 414 exhibited larger volumetric strains due to their greater thermal sensitivity. 415 These results highlight that higher plasticity leads to greater plastic deformation in normally consolidated states, while increasing OCR progressively reduces permanent contraction, transitioning to predominantly elastic behavior at higher OCR levels. These trends are consistent with the literature (Baldi et al., 1988; Hueckel and Borsetto, 1990; AbuelNaga et al., 2007b; Di Donna et al., 2016; Shetty et al., 2019; Sultan et al., 2002; Takai et al., 2016; Vega and McCartney, 2015).

6.2.1. OCR role

In order to conduct a comprehensive analysis of the volumetric response 424 of fine-grained soils to thermal loading, a diverse dataset is essential. Figure 9 integrates data from both LP and HP Samples and various literature sources. 427

As expected, the data reveal that NC samples exhibit plastic contrac-

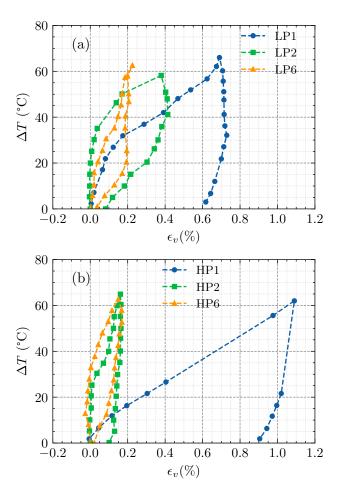


Figure 8: Thermal volume change after drained heating and cooling for (a) Low Plasticity (LP) and (b) High Plasticity (HP) samples, illustrating the differences in contraction and elastic behavior across OCRs.

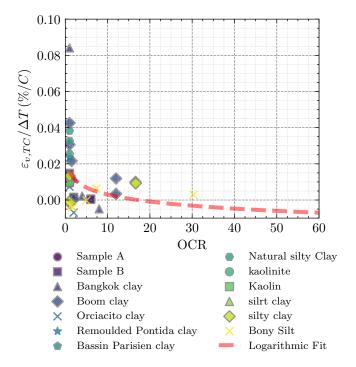


Figure 9: Volumetric strain after heating-cooling cycle for various clay types from literature and this research, including Bangkok clay (AbuelNaga et al., 2007a), Boom clay (Baldi et al., 1988), Remoulded Pontida clay (Hueckel and Pellegrini, 2002), Bassin Parisien clay (Robinet et al., 1996), Natural silty clay (Shetty et al., 2019), Kaolinite (Shetty et al., 2019), Silt clay (Di Donna and Laloui, 2015), and Bony silt (Vega and McCartney, 2015).

tion, consistent with higher strain values. As OCR increases, plastic deformation diminishes. Moderately over-consolidated soils (OCR \geq 2 and < 6) show reduced plastic strain, while highly over-consolidated soils (OCR \gg 6) transition to elastic-dominated behavior, exhibiting minimal volumetric change. These trends confirm the stabilizing effect of higher OCR, with overconsolidation limiting plastic deformation and promoting elastic recovery.

However, variations in soil-specific characteristics, such as plasticity, mineralogy, and *stress history* (e.g., unloading vs. reloading), can influence behavior. Some soils in the OCR range of 6–16 display deviations, such as transient elastic expansion or immediate contraction upon heating. These details underscore that OCR alone does not fully capture the complexity of thermal volume change.

To describe the relationship between normalized volumetric strain and OCR, a logarithmic trendline was fitted to the data. This trendline indicates that, for OCR values greater than zero, the thermally induced volumetric strain $\epsilon_{v,TC}$ can be approximated by the function

$$\epsilon_{v,TC} = -0.0052\Delta T \ln(\text{OCR}) + 0.0175\Delta T, \tag{2}$$

where the coefficients -0.0052 and 0.0175 were determined via leastsquares regression. The trendline indicates that increasing OCR reduces
normalized thermal strain, transitioning from high plastic contraction at low
OCR to minimal strain at high OCR. Scatter around the trendline reflects the

influence of additional factors such as soil fabric, sand content, and plasticity.
This fitted equation highlights the stabilizing effect of over-consolidation on
the thermally induced strain, providing a useful framework for understanding
the behavior of fine-grained soils under thermal cycling.

6.2.2. Plasticity role (PI)

Building on the preceding discussion regarding the pivotal role of the overconsolidation ratio (OCR) in governing thermally induced volume changes, attention now shifts to plasticity as a further critical parameter in the thermal behavior of fine-grained soils. Figure 10 offers a comprehensive examination of how the plasticity index (PI) influences the volumetric strain ($\epsilon_{v,TC}$) in normally consolidated soils subjected to a full heating-cooling cycle, thereby underscoring the importance of intrinsic soil properties in shaping thermal responses.

A central finding derived from this figure is the robust positive correlation between plasticity and volumetric strain. In particular, soils with higher plasticity indices exhibit markedly greater contraction or deformation under temperature fluctuations. This observation highlights the dominant impact of plasticity in determining the extent of thermally induced volume change, often exceeding even the effect of OCR. For instance, the LP sample (with PI = 16) undergoes comparatively modest thermal contraction, whereas the HP sample (with PI = 35) experiences notably higher strain, illustrating the pronounced influence of increased plasticity on soil deformation. Similarly, data from Bangkok clay (PI = 60) (AbuelNaga et al., 2007b) and Natural silty clay (PI = 41) (Shetty et al., 2019) confirm this trend, demonstrating elevated strain values associated with high plasticity.

In order to quantify this relationship, an exponential trendline was fitted to the data in Figure 10, yielding the following equation:

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where $\epsilon_{v,TC}$ is the volumetric strain after the thermal cycle, ΔT denotes

$$\epsilon_{v,TC} = 0.0057\Delta T \cdot e^{0.0394PI} \tag{3}$$

the temperature increment, and PI represents the plasticity index. This equation reveals that volumetric strain escalates with increasing PI, and it shows strong congruence with both the current dataset and previous findings in the literature, such as Boom clay NC (PI = 30) (Robinet et al., 1996) and Remoulded Pontida clay (PI = 12.9) (Hueckel and Pellegrini, 2002). 480 A further analysis of plasticity index ranges elucidates several notable 481 tendencies. Low-PI soils (i.e., PI < 25), such as LP sample and Kaolin 482 (PI = 24) (Shetty et al., 2019), demonstrate relatively minimal thermal 483 volumetric strain, reflecting a modest sensitivity to temperature changes. 484 Moderate-PI soils ($25 \le PI < 40$) begin to exhibit a gradual increase in 485 thermally induced strain, marking a transitional zone. For instance, Bassin 486 Parisien clay (PI = 20) (Robinet et al., 1996) and Boom clay (PI = 30)487 (Baldi et al., 1988) illustrate the moderate thermal responses observed in 488 this range. In contrast, High-PI soils (PI ≥ 40), exemplified by Bangkok

clay (PI = 60) (AbuelNaga et al., 2007b) and Natural silty clay (PI = 41)(Shetty et al., 2019), undergo pronounced thermal contraction, presumably 491 due to their greater capacity for particle rearrangement and water retention. The threshold at which a pronounced shift in behavior occurs lies in 493 the range of approximately PI = 35-40. Below this boundary, volumetric strain typically remains comparatively moderate, whereas soils surpassing 495 this limit exhibit significantly intensified contraction under heating-cooling 496 cycles. Although some scatter persists—owing to variations in soil mineralogy, fabric, and stress history—the overarching pattern confirms plasticity as a key determinant of soil behavior under thermal loading. For example, Bony Silt (PI = 4) (Vega and McCartney, 2015) remains an outlier, showcasing minimal strain even at very low plasticity, further supporting the need for 501 site-specific investigations.

Taken collectively, these results underscore the necessity of considering 503 the plasticity index in the development and enhancement of thermomechani-504 cal constitutive models. While current constitutive models within the critical 505 state framework already account for the role of OCR in influencing the volu-506 metric behavior of clays under thermal loading, they do not yet incorporate 507 the influence of the plasticity index. Future advancements in constitutive 508 modeling must address this gap to enhance the predictive capability of existing frameworks and account for the observed correlations between plasticity, 510 OCR, and volumetric strain under thermal loading.

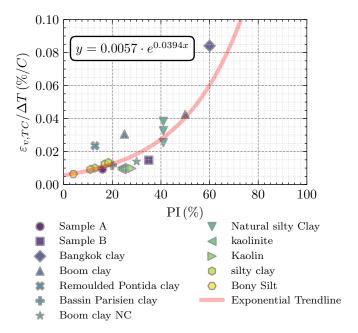


Figure 10: effect of plasticity on volumetric strain after heating-cooling cycle for normally consolidated samples from literature and this research, including Bangkok clay (AbuelNaga et al., 2007b), Boom clay (Baldi et al., 1988; Robinet et al., 1996; Sultan et al., 2002), Remoulded Pontida clay (Hueckel and Pellegrini, 2002), Bassin Parisien clay (Robinet et al., 1996), Natural silty Clay (Shetty et al., 2019), kaolinite (Shetty et al., 2019), silt clay (Di Donna and Laloui, 2015), and Bony Silt (Vega and McCartney, 2015).

6.2.3. The combined roles of OCR and PI

To explore the complex thermal volumetric behavior of overconsolidated soils, Figures 11 and 12 focus on two critical parameters: the overconsolidation ratio (OCR) and the plasticity index (PI). In these figures, the thermal volumetric strain ($\epsilon_{v,TC}$) is normalized by the temperature increment (ΔT) and either OCR or PI, providing a consistent basis for cross-sample comparison. This normalization ensures that differences in thermal loading and inherent soil plasticity do not mask key behavioral trends, allowing a clearer assessment of how OCR and PI each affect thermal response.

Figure 11 highlights the influence of OCR, plotted on a logarithmic scale 521 to accommodate the broad range of values. The general trend suggests that soils with relatively low OCR are prone to thermal contraction, whereas 523 those with higher OCR tend toward thermal expansion, in line with the 524 idea that more heavily overconsolidated soils resist volumetric change. The fitted regression line (using log(OCR)) shows an R^2 of about 0.226—indicating a moderate but not definitive predictive capability—and a negative 527 slope for log(OCR). This negative slope implies a decrease in the normalized 528 strain measure as OCR increases, consistent with a shift from contractiondominated to expansion-dominated behavior. The scatter in the data, however, underscores that OCR alone cannot fully explain thermal volumetric changes, likely due to additional factors such as plasticity and mineral composition.

Figure 12 examines the effect of PI, with thermal volumetric strain nor-

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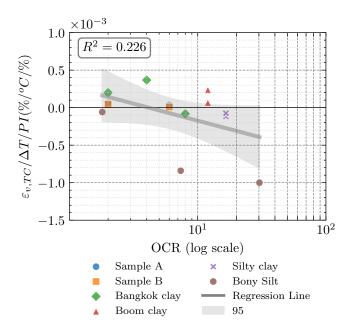


Figure 11: Effect of OCR on normalized volumetric strain for overconsolidated samples, including data from this study (Silty clay, Bony Silt) and literature (Bangkok clay (Abuel-Naga et al., 2007b), Boom clay (Sultan et al., 2002), Silty clay (Di Donna et al., 2016), Bony Silt (Vega and McCartney, 2015)).

malized by ΔT and OCR. Compared to OCR, PI offers a similarly moderate predictive power (with an R^2 of about 0.212). The positive slope in the re-536 gression model indicates that the normalized strain measure increases with 537 PI. In practical terms, however, high-PI soils typically experience more pro-538 nounced contraction, especially under thermal loading. This apparent dis-539 connect between the slope and the observed contraction can stem from how 540 strain normalization and other soil-specific factors (e.g., structure, mineral-541 ogy) interplay with PI in the statistical fit. Despite that nuance, soils like highly plastic Bangkok clay (PI $\approx 60\%$) reliably show considerable thermal contraction, whereas low-PI soils (e.g., Bony Silt, PI = 4%) exhibit minimal or slightly expansive tendencies under similar conditions.

While neither regression achieves a high level of explanatory power, the results confirm that both OCR and PI significantly affect thermal volumetric strain. The following simplified models encapsulate the relationships:

$$\frac{\epsilon_{v,TC}}{\Delta T \cdot \text{PI}} = A \log(\text{OCR}) + B \quad \text{(Figure 11)}$$

$$\frac{\epsilon_{v,TC}}{\Delta T \cdot \text{OCR}} = C \,\text{PI} + D \quad \text{(Figure 12)} \tag{5}$$

Here, A, B, C, and D are regression coefficients obtained from curvefitting. Although the data shows that OCR can delineate the transition between contraction and expansion, PI remains an equally important predictor of thermal response. Indeed, high-PI soils may still undergo notable contraction even at high OCR, highlighting the need to account for plasticity when modeling thermal soil behavior. Consequently, conventional models that focus primarily on OCR should incorporate PI for better accuracy in geotechnical design, especially for scenarios involving thermally induced volume change.

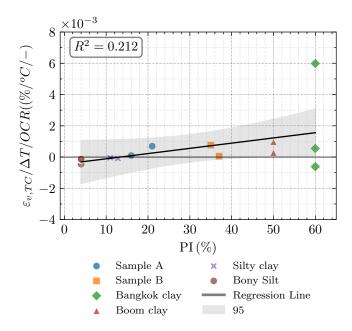


Figure 12: effect of plasticity on normalized volumetric strain for overconsolidated samples, including data from this study (Silty clay, Bony Silt) and literature (Bangkok clay (AbuelNaga et al., 2007b), Boom clay (Sultan et al., 2002), Silty clay (Di Donna et al., 2016), Bony Silt (Vega and McCartney, 2015)).

8 6.2.4. Thermal Cyclic behavior

The response of the samples to repeated thermal cycling was investigated by subjecting them to drained heating and cooling cycles while maintaining a constant effective stress. Figure 13a illustrates the volumetric strain response of LP1. The initial heating phase results in a significant contraction,
while the subsequent cooling phase shows partial recovery, with a noticeable
plastic deformation evident at the end of the first cycle. Further heating and
cooling cycles lead to progressively smaller volumetric changes, with the overall contraction diminishing with each cycle. This pattern suggests that the
sample undergoes a decreasing rate of plastic deformation, moving towards a
more elastic behavior as the number of thermal cycles increases. Figure 13b
presents the volumetric strain behavior of sample LP6. In contrast to the normally consolidated sample, the strain response of the overconsolidated sample
is comparatively smaller. Additionally, the extent of contraction during the
heating phases and the partial recovery during cooling phases is considerably
less when compared to 13a.

Figure 14 presents the volumetric strain responses of Sample HP under different overconsolidation ratios (OCR) when subjected to repeated drained heating and cooling cycles. Figure 14a depicts the behavior of a normally consolidated sample (HP1) under thermal cycling. The sample exhibits similar contraction during heating phases and partial recovery during cooling phases, as seen in the normally consolidated sample of LP1, as discussed previously. As the number of thermal cycles increases, the changes in volumetric strain become less pronounced, indicating a reduction in plastic deformation and a trend toward stabilization. Figure 14b illustrates the thermal cycling response of a moderately overconsolidated Sample B (HP2). The volumetric strain range is markedly reduced compared to the normally consolidated

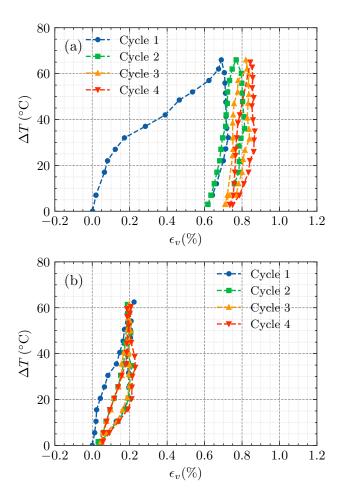


Figure 13: Thermal-induced plastic volumetric strain (ϵ_v) versus temperature change (ΔT) for (a) LP1 and (b) LP6 samples under cyclic thermal loading.

case. The overall contractive behavior reduces substantially for each subsequent thermal cycle, and the response seems largely elastic. Figure 14c
shows the results for a highly over-consolidated HP sample (HP6). The
overall strain variations are significantly reduced when compared with the
previously discussed cases. The sample exhibits near-elastic behavior from
the outset, showing immediate and almost complete recovery of deformation
from the initial heating cycle with negligible residual volume change.

The contraction and recovery phases show that a significant degree of stabilization occurs after the first thermal cycle. Furthermore, it seems that thermal strain tends to stabilize after the 4th thermal cycle, and soils tend to enter elastic behavior even in the normally consolidated state. These results highlight the critical role of overconsolidation in influencing the thermal response of fine-grained soils. As the OCR increases, the tendency for irreversible plastic deformation under thermal cycling decreases. Higher values of OCR seem to enhance the elastic response, while the magnitude of volume change becomes increasingly suppressed, suggesting the influence of stress history on the structure and behavior of the tested material.

To quantify the progressive accumulation of volumetric strain resulting from repeated thermal cycling, Tables 5 and 6 provide a detailed breakdown of the induced volumetric strains at the end of each heating and cooling phase for the LP sample and HP sample, respectively, under different initial OCR conditions. As illustrated previously in Figures 13–14, the overall volume change and rate of plastic deformation for a given soil was closely influenced

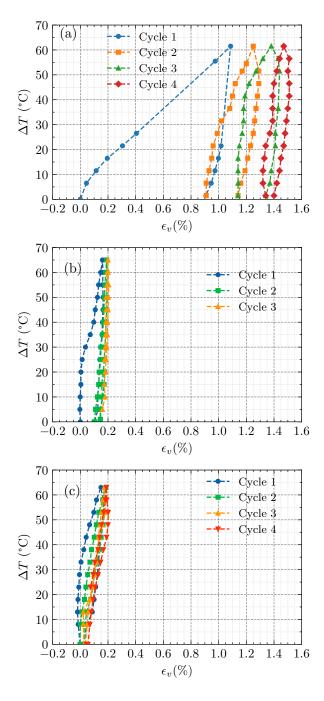


Figure 14: Thermal-induced plastic volumetric strain (ϵ_v) versus temperature change (ΔT) for the HP1, HP2, and HP6 samples under cyclic thermal loading.

by OCR. These values complement that information with quantitative measurements. Table 5 outlines the accumulating strains for the LP sample (low plasticity clay) through four thermal cycles, contrasting a normally consolidated sample (OCR=1) against an overconsolidated sample (OCR=6). A consistent contractive trend is apparent upon every heating and subsequent recovery of some of the volume in every cooling step of the process, with magnitudes gradually reducing after each cycle.

Table 5: Accumulating strain, $\varepsilon_v(\%)$ during heating-cooling cycles for LP sample.

Cycle	Process	OCR=1	OCR=6
1	heating	0.69	0.22
	cooling	0.62	0.04
2	heating	0.77	0.19
	cooling	0.71	0.05
3	heating	0.82	0.20
	cooling	0.73	0.05
4	heating	0.84	0.21
	cooling	0.74	0.05

Table 6 presents the accumulating strains for HP sample (high plasticity clay) across four thermal cycles, contrasting normally consolidated (OCR=1), and an overconsolidated state (OCR=2 & 6). A more significant volumetric response is observed in this sample when compared to the low plasticity material in Table 5 for OCR=1 samples, which is also discussed earlier with regard to Figure 14. While the trends of the thermal loading cycles in the HP sample under different OCR conditions align with the trends of the LP sample in their corresponding OCR conditions, it is observed that the overconsolidated

samples with a higher OCR show negligible overall changes after one thermal cycle compared to a considerably reduced accumulated strain after multiple cycles when the sample had a lower OCR condition.

Table 6: Accumulating strain, $\varepsilon_v(\%)$ during heating-cooling cycles for HP sample.

Cycle	Process	OCR=1	OCR=2	OCR=6
1	heating	1.09	0.16	0.15
	cooling	0.91	0.10	0.02
2	heating	1.24	0.18	0.16
	cooling	1.14	0.14	0.05
3	heating	1.37	0.20	0.16
	cooling	1.33	0.16	0.07
4	heating	1.47		0.19
	cooling	1.40		0.07

In both sample groups, the cumulative plastic deformation during each 626 successive heating cycle decreases, as each subsequent cooling cycle only re-627 stores some volume back to the sample, and the rate of plastic deformation 628 noticeably reduces after each cycle. Figure 15, representing data from the 620 present study combined with a comparable dataset available in published 630 form, demonstrates that plastic components of volumetric strains are most 631 prominent during the early cycles of repeated drained heating on normally 632 consolidated samples, where material-specific changes (linked with Plasticity Index), exhibit notable variations in the magnitude of deformations at first phases followed by reduced change rates after subsequent phases.

Data generally confirms reduced changes over thermal treatment through further cycling for every test series used, irrespective of material type or

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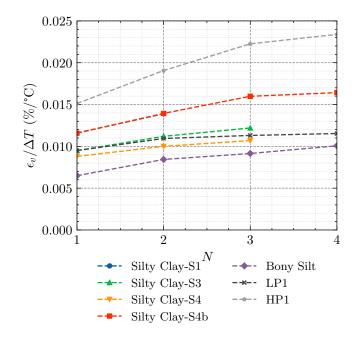


Figure 15: Relationship between the normalized plastic strain increment per degree Celsius and the number of thermal cycles (N) for various soil types. The plot compares silty clays (S1, S3, S4, S4b) from (Di Donna and Laloui, 2015), Bony Silt from (Vega and McCartney, 2015), and two newly tested samples (Samples LP1 and HP1).

values showing reduction and trend toward an elastic like behavior with progressive cycle number but mostly being dominant when plasticity properties are high. The results indicate that thermal volume change stabilizes after the 4th thermal cycle or before the 5th cycle. Furthermore, the incremental plastic deformation during each heating-cooling cycle appears to increase with higher plasticity.

7. Conclusions

This research provides a comprehensive experimental investigation into the thermally induced volumetric behavior of fine-grained soils, focusing on the coupled effects of over-consolidation ratio (OCR), plasticity, and stress history. Key findings include:

- Stress History Dominance over OCR in Over-Consolidated

 Clays: Contrary to expectations based solely on OCR, the recent stress

 history, specifically reloading, significantly influenced the behavior of

 over-consolidated samples. Reloaded clays contracted upon heating

 rather than undergoing initial expansion. This indicates that current

 models emphasizing only OCR for thermal volumetric change are in
 adequate.
- Plasticity as a Key Driver of Volumetric Strain: The plasticity
 index (PI) was revealed as a pivotal parameter. High-plasticity clays
 exhibited substantially larger plastic contractions, greater total volu-

- metric changes, and slower stabilization rates under thermal cycling compared to low-plasticity clays with the same OCR values. These observations underscore that material-specific parameters dominate the magnitude and rates of volume change.
- Quantified Relationships with OCR and PI: Logarithmic relationships were quantified to express volumetric strain as a function of temperature change, OCR and PI.

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- Repeated Thermal Cycles Stabilisation: Repeated thermal cycles induced a gradual decrease in cumulative plastic strain, highlighting that a major portion of the total thermal change was accrued within the first few cycles (less than five thermal cycles) in most samples. Clays trended toward a more elastic behavior with increased cycle counts. Overconsolidated soils stabilized more quickly than their normally consolidated counterparts and showed greater elastic recovery.
- Interactive Effects of OCR, PI, and Loading Path: Data syn-673 thesis demonstrates that thermal behavior is governed by an interplay 674 between OCR, PI, and recent stress path (reloading or unloading), 675 contradicting conventional assumptions that OCR is the dominant pa-676 rameter. The study highlights that PI has a significant influence in 677 modifying the contraction of highly over-consolidated soils. Even with 678 very high OCRs, high PI soils can exhibit notable contraction, while 679 the thermal sensitivity of soils at low OCR depends heavily on PI. 680

Findings from this study provide compelling evidence for reevaluating existing thermo-mechanical constitutive models to specifically include plasticity alongside stress history (including the effects of reloading) for improved predictive capability in thermally active geo-energy applications. Future research should utilize these findings to refine constitutive models for better prediction of soil behavior in diverse real-world scenarios.

The insights gained from our thermo-mechanical tests have practical ap-687 plications for geologic risk assessments in various subsurface engineering projects. Clays subjected to cyclic heating (e.g., in geothermal energy extraction or seasonal underground thermal energy storage systems) may undergo progressive deformation or develop thermally induced microcracks, potentially altering permeability and mechanical strength. This phenomenon becomes particularly relevant in regions with clay-rich strata similar to those of District 8 in Budapest, where high-plasticity minerals magnify thermally triggered volumetric changes. By matching the OCR- and PI-dependent strain models with specific lithologies, site engineers can more reliably ac-696 count for the cumulative effects of temperature during facility operations. 697 This expanded viewpoint not only mitigates the potential for unexpected 698 ground movements but also refines the safety margins necessary for struc-699 ture integrity in the subsurface. Overall, our experimental outcomes bridge 700 the gap between small-scale laboratory characterizations and large-scale geo-701 logic risk management, particularly where thermally active systems interact with fine-grained formations.

704 References

- Abuel-Naga, H. M., Bergado, D. T., Chaiprakaikeow, S., 2006a. Innovative
- thermal technique for enhancing the performance of prefabricated vertical
- drain during the preloading process. Geotextiles and Geomembranes 24 (6),
- 708 359–370.
- AbuelNaga, H. M., Bergado, D. T., Ramana, G. V., Grino, L., Rujivipat, P.,
- Thet, Y., 2006b. Experimental evaluation of engineering behavior of soft
- Bangkok clay under elevated temperature. Journal of Geotechnical and
- Geoenvironmental Engineering 132 (7), 902–910.
- AbuelNaga, H., Bergado, D., Bouazza, A., Ramana, G., 2007a. Volume
- change behaviour of saturated clays under drained heating conditions:
- experimental results and constitutive modeling. Canadian Geotechnical
- Journal 44 (8), 942–956.
- AbuelNaga, H. M., Bergado, D. T., Ramana, G. V., 2007b. Thermal consol-
- idation of soft Bangkok clay. Soils and Foundations 47 (3), 423–432.
- ALJUNDI, K., Figueiredo, A., Vieira, A., Lapa, J., Cardoso, R., 2024.
- Geothermal energy system application: From basic standard performance
- to sustainability reflection. Renewable Energy 220, 119612.
- Armand, G., Conil, N., Talandier, J., Sevedi, D. M., 2017. Fundamental as-
- pects of the hydromechanical behaviour of Callovo-Oxfordian claystone:

- from experimental studies to model calibration and validation. Computa-
- tional Geotechnics 85, 277–286.
- Aresti, L., Christodoulides, P., Florides, G., 2018. A review of the design
- aspects of ground heat exchangers. Renewable and Sustainable Energy
- Reviews 92, 757–773.
- Baldi, G., Hueckel, T., Pellegrini, R., 1988. Thermal volume changes of the
- mineral-water system in low-porosity clay soils. Canadian Geotechnical
- Journal 25 (4), 807–825.
- Ballarini, E., Graupner, B., Bauer, S., 2017. Thermal-hydraulic-mechanical
- behavior of bentonite and sand-bentonite materials as seal for a nuclear
- waste repository: Numerical simulation of column experiments. Applied
- 735 Clay Science 135, 289–299.
- Bumbieler, F., Plúa, C., Tourchi, S., Vu, M. N., Vaunat, J., Gens, A.,
- Armand, G., 2021. Feasibility of constructing a full-scale radioactive
- high-level waste disposal cell and characterisation of its thermo-hydro-
- mechanical behavior. International Journal of Rock Mechanics and Mining
- ⁷⁴⁰ Sciences 137, 104555.
- Burghignoli, A., Desideri, A., Miliziano, S., 2000. A laboratory study on
- the thermomechanical behaviour of clayey soils. Canadian Geotechnical
- Journal 37 (4), 764–780.

- Campanella, R. G., Mitchell, J. K., 1968. Influence of temperature varia-
- tions on soil behavior. Journal of Soil Mechanics and Foundations Division
- ⁷⁴⁶ 94 (3), 709–734.
- Cekerevac, C., Laloui, L., 2004. Experimental study of thermal effects on the
- mechanical behaviour of a clay. International Journal for Numerical and
- Analytical Methods in Geomechanics 28 (3), 209–228.
- 750 Chen, W. Z., Ma, Y. S., Yu, H. D., Li, F. F., Li, X. L., Sillen, X., 2017. Effects
- of temperature and thermally-induced microstructure change on hydraulic
- conductivity of Boom Clay. Journal of Rock Mechanics and Geotechnical
- ⁷⁵³ Engineering 9 (3), 383–395.
- Coccia, C. J. R., McCartney, J. S., 2016. Thermal volume change of poorly
- draining soils I: Critical assessment of volume change mechanisms. Com-
- puters and Geotechnics 80, 26–40.
- Cui, Y., 2022. Soil-atmosphere interaction in earth structures. Journal of
- Rock Mechanics and Geotechnical Engineering 14 (1), 35–49.
- Cui, Y. J., Sultan, N., Delage, P., 2000. A thermomechanical model for
- saturated clays. Canadian Geotechnical Journal 37 (3), 607–620.
- Delage, P., Sultan, N., Cui, Y. J., Pascal, A. B., 2000. On the thermal
- consolidation of boom clay. Canadian Geotechnical Journal 37, 343–354.
- Di Donna, A., Laloui, L., 2015. Response of soil subjected to thermal cyclic

- loading: experimental and constitutive study. Engineering Geology 190, 65–76.
- Di Donna, A., Ferrari, A., Laloui, L., 2016. Experimental investigations of the
- soil-concrete interface: physical mechanisms, cyclic mobilisation, and Be-
- haviour at different temperatures. Canadian Geotechnical Journal 53 (4),
- 769 659-672.
- Faizal, M., Bouazza, A., Haberfield, C., McCartney, J. S., Asce, M., 2018.
- Axial and radial thermal responses of a field-scale energy pile under mono-
- tonic and cyclic temperature changes. Journal of Geotechnical and Geoen-
- vironmental Engineering 144 (10), 1–14.
- Favero, V., Ferrari, A., Laloui, L., 2016. Thermo-mechanical volume change
- behaviour of Opalinus Clay. International Journal of Rock Mechanics and
- Mining Sciences 90, 15–25.
- Gary, H., 1936. Progress report on the consolidation of fine-grained soils. In:
- First International Conference on Soil Mechanics and Foundation Engi-
- neering. Harvard University Press, pp. 138–141.
- Gens, A., Sánchez, M., Guimarães, L. Do N., Alonso, E. E., Lloret, A.,
- Olivella, S., Villar, M. V., Huertas, F., 2009. A full-scale in situ heat-
- ing test for high-level nuclear waste disposal: observations, analysis and
- interpretation. Géotechnique 59 (4), 377–399.

- Gong, W., Zhang, S., Juang, C. H., Tang, H., Pudasaini, S. P., 2024. Displacement prediction of landslides at slope-scale: Review of physics-based and data-driven approaches. Earth-Science Reviews 258, 104948.
- Hamidi, A., Shirasb, A., Tourchi, S., 2024. On the thermal failure of sand-clay mixture. Proceedings of the XVIII European Conference on Soil Mechanics and Geotechnical Engineering (ECSMGE 2024), London, United Kingdom.
- Hoseinimighani, H., Szendefy, J., 2022. A Review on mechanisms of thermally induced volume changes in fine soil. Minerals 572 (12), 2075-163X.
- H. Hoseinimighani, S. Tourchi, and J. Szendefy, 2023. Thermomechanical behaviour of silty sandy clays: An experimental and numerical investigation. International Society for Soil Mechanics and Geotechnical Engineering (ISSMGE), London, United Kingdom.
- Hueckel, T., Borsetto, M., 1990. Thermoplasticity of saturated soils and shales: constitutive equations. Journal of Geotechnical Engineering 116 (12), 1765–1777.
- Hueckel, T., Pellegrini, R., 2002. Reactive plasticity for clays: application to a natural analog of long-term geomechanical effects of nuclear waste disposal. Engineering Geology 64 (2–3), 195–215.
- 802 Israelachvili, J. N., 1991. Intermolecular and surface forces. Academic Press.
- Jabbarzadeh, M., Sadeghi, H., Tourchi, S., Darzi, A. G., 2024. Thermohydraulic analysis of desiccation cracked soil strata considering ground

- temperature and moisture dynamics under the influence of soil-atmosphere
- interactions. Geomechanics for energy and the Environment 38, 100558.
- Laloui, L., Nuth, M., Vulliet, L., 2006. Experimental and numerical inves-
- tigations of the behaviour of a heat exchanger pile. International Journal
- for Numerical and Analytical Methods in Geomechanics 30 (8), 763–781.
- Liu, W., Xu, M., 2017. 2D axisymmetric model research of helical heat ex-
- changer inside pile foundations. Procedia Engineering 205, 3503–3510.
- Liu, J., Zhou, C., Tang, A. M., 2024. Thermo-hydro-mechanical behaviour
- of energy tunnel in unsaturated soils. Tunnelling and Underground Space
- 814 Technology 153, 106012.
- Melchiorre, C., Frattini, P., 2011. Modelling probability of rainfall-induced
- shallow landslides in a changing climate, Otta, Central Norway. Climatic
- change 113 (2), 287–306.
- Mohajerani, M., Delage, P., Sulem, J., Monfared, M., Tang, A. M., Gatmiri,
- B., 2014. The thermal volume changes of the Callovo-Oxfordian claystone.
- Rock Mechanics and Rock Engineering 47 (1), 131–142.
- 821 Ng, C. W. W., Gunawan, A., Shi, C., Ma, Q. J., Liu, H. L., 2016. Centrifuge
- modelling of displacement and replacement energy piles constructed in
- saturated sand: a comparative study. Géotechnique Letters 6 (1), 34–38.
- Paaswell, R. E., 1967. Temperature effects on clay soil consolidation. Journal
- of the Soil Mechanics and Foundations Division 93 (3), 9–22.

- Plum, R. L., Esrig, M. I., 1969. Some temperature effects on soil compressibil-
- ity and pore pressure. Transportation Research Board / Highway Research
- Board Special Report (103).
- Pothiraksanon, C., Bergado, D. T., Abuel-Naga, H. M., 2010. Full-scale em-
- bankment consolidation test using prefabricated vertical thermal drains.
- 831 Soils and Foundations 50 (5), 599–608.
- Robinet, J. C., Rahbaoui, A., Plas, F., Lebon, P., 1996. A constitutive ther-
- momechanical model for saturated clays. Engineering Geology 41 (1-4),
- 834 145–169.
- Robinet, J.-C., Pasquiou, A., Jullien, A., Belanteur, N., Plas, F., 1997. Ex-
- periences de laboratoire sur le comportement thermo-hydro-mécanique de
- matériaux argileux remaniés gonflants et non gonflants. Revue Française
- de Géo-technic 81, 53–80.
- 839 Sadeghi, H., Jabbarzadeh, M., Tourchi, S., 2024. Thermo-hydro-mechanical
- modelling of the heterogeneous subsidence and swelling in the desiccation
- cracked clayey strata. Engineering Geology 343, 107798.
- 842 Scaringi, G., Loche, M., 2022. A thermo-hydro-mechanical approach to soil
- slope stability under climate change. Geomorphology 401, 108108.
- Scaringi, G., Loche, M., Tourchi, S., Lombardo, L., 2022. Temperature and
- slope stability in temperate climate. In: 17th plinius conference on mediter-
- ranean risks.

- Shetty, R., Singh, D. N., Ferrari, A., 2019. Volume change characteristics of fine-grained soils due to sequential thermomechanical stresses. Engineering Geology 253, 47–54.
- Shibasaki, T., Matsuura, S., Hasegawa, Y., 2017. Temperature-dependent residual shear strength characteristics of smectite-bearing landslide soils. Journal of Geophysical Research: Solid Earth 122 (2), 1449–1469.
- Sultan, N., Delage, P., Cui, Y. J., 2002. Temperature effects on the volume change behaviour of Boom clay. Engineering Geology 64 (2–3), 135–145.
- Takai, A., Ghaaowd, I., McCartney, J. S., Katsumi, T., 2016. Impact of drainage conditions on the thermal volume change of soft clay. In: Geo-Chicago 2016. American Society of Civil Engineers (ASCE), pp. 32–41.
- TIAN, X., Li, T., Gong, W., 2023. Temperature effect on the stability of
 3D reinforced slopes under unsaturated flow conditions. Computers and
 Geotechnics 164, 105771.
- Tourchi, S., Vaunat, J., Gens, A., Bumbieler, F., Vu, M. N., Armand, G., 2021. A full-scale in situ heating test in Callovo-Oxfordian claystone:
 observations, analysis and interpretation. Computational Geotechics 133,
 104045.
- Tourchi, S., Jabbarzadeh, M., Lavasan, A. A., Sadeghi, H., Racek, O., 2024. Thermo-hydro-mechanical dynamics of a rock slope: Integrated

- field and numerical analysis at the Požáry test site in the Czech Repub-
- lic. Journal of Rock Mechanics and Geotechnical Engineering, in press,
- S1674775524005560.
- Towhata, I., Kuntiwattanaku, P., Seko, I., Ohishi, K., 1993. Volume change
- of clays induced by heating as observed in consolidation tests. Soils and
- Foundations 33 (4), 170–183.
- Vega, A., McCartney, J. S., 2015. Cyclic heating effects on thermal volume
- change of silt. Environmental Geotechnics 2 (5), 257–268.

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Declaration of interests

☐The authors declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

⊠The authors declare the following financial interests/personal relationships which may be considered as potential competing interests:

Saeed Tourchi reports financial support was provided by University of Luxembourg. If there are other authors, they declare that they have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.